## VARIOMAX

Soffit formwork with H 20 timber beams Instructions for erection and use


## HUNNEBECK

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### 1.0 Product features

In combination with tubular steel props, tripod stands, fork heads and shuttering panels, the H 20 beams provide flexible, yet costeffective soffit formwork for any ground-plan, slab thickness and room height.

The H 20 timber beam is especially practical due to its low weight ( $5.0 \mathrm{~kg} / \mathrm{m}$ ), its good statical figures, and its exacting workmanship in details.

A very long duration of life is assured by its high-grade bonding and its rounded beam ends.
The H 20 timber has a general approval by the building supervisory board.

Important notes:
The following instructions for erection and use include detailed information on the handling and proper application of the products that are described and depicted. All instructions regarding technical operation and function have to be observed carefully. Exceptional use requires a separate design calculation.
With regard to safe and technically correct use of our products abroad, all relevant safety rules, regulations and safety instructions of national institutes and/or local authorities have to be followed.

Generally, only flawless material must be used.

Damaged components have to be sorted out. In case
of repairs, only original spare
parts of the HÜNNEBECK
Company must be used.
Combined use of our
formwork system with equipment from other suppliers may involve certain dangers and, therefore, requires an additional checkup.

For reasons of further technical development we emphatically reserve the right to revise, change or modify any of the product's components at any time without prior notice.



## Quick lowering mechanism:

Another special feature is that all the props are equipped with the patented quick-release bolt, which, with a blow of the hammer, immediately releases the adjustment nut.


Practical accessory items make Variomax soffit formwork even faster and more efficient. The tripod stand, for example, makes it easier to erect the props.

Striking is made easier by lowering the shuttering plane by about 6 cm using the adjustment nut of the steel props. With the space which results from the first lowering and by tilting the timber beams, the shuttering material can be systematically removed,
while going easy on materials.
(Subject to technical modifications.)

### 3.0 Components

| Art. No | Weight <br> kg/pcs. |
| :--- | :--- | :--- |

## Beam Formwork



## Joist clamping connector

Can be fastened to any timber beam like H 20 and R 24 with 8 cm wide and at maximum 6 cm thick flanges (see page 12-16).


## Accessories



## Tripod stand

Simplifies the erection of Europlus and AS props for soffit formwork assembly.
Only to be used as an erection aid; does not replace the stiffening necessary for the supporting structure.


## Fork head $\mathbf{8 / 2 0}$

Protects H 20 beams which serve as primary beams from falling. Holds either 1 or 2 beams. Is secured in the prop with the T-bolt.
(Order $1 \times \mathrm{T}$-bolt for this purpose.)

| Art. No |
| :--- |
| Nescription |



3-S-Shuttering panels 200 (Package $100 \mathrm{~m}^{2}$ ) According to DIN 18215.
Thickness $21 \mathrm{~mm}, \mathrm{~L}=2.00 \mathrm{~m}, \mathrm{~W}=0.50 \mathrm{~m}$.

## Assembly fork

Simplifies the laying and removal of H 20 shuttering beams.

## Assembly fork

Simplifies the laying and removal of H 20 shuttering beams (see page 9).

## Euro stacking frame 120/80

Steel, hot-dip galvanized
Load-bearing capacity: 1200 kg
For storing and transporting scaffolding and formwork materials. A maximum of 6 stands can be stacked.

## Euro stacking frame 120/80

Steel, hot-dip galvanized
Load-bearing capacity: 1200 kg
For storing and transporting scaffolding and formwork materials. A maximum of 6 stands can be stacked (see page 19).


3-S-Shuttering panels 150 (Package $75 \mathrm{~m}^{2}$ ) According to DIN 18215.
Thickness $21 \mathrm{~mm}, \mathrm{~L}=1.50 \mathrm{~m}, \mathrm{~W}=0.50 \mathrm{~m}$.


## Mobile-set

To manoeuvre the Euro stacking frame.
Is simply slotted into the stacking frame, which can then even be moved when loaded (Working load: $1,300 \mathrm{~kg}$ ).
See page 19.

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### 4.0 Use and erection

## Prop with fork head $\mathbf{8 / 2 0}$

To begin with, the fork heads 8/20 have to be attached to the tubular steel props.
The inserted heads are secured with a T-bolt.

Both fork heads have a 2-way design. This means that in one position one timber beam, and in the other position - a $90^{\circ}$ rotation - two timber beams can be inserted in the head (assuming a 8 cm beam width).

## Prop with tripod stand

The tripod stand makes it easier to set up the tubular steel props during erection. The prop is simply set in the open stand and secured through the clamping loop with a gentle blow of the hammer. The tripod stand can be used with all types of props. The flexibly mounted supporting legs of the tripod stand permit an optimal fit, even in the corners of the structure.

## Important note

After the soffit formwork has been completely erected in the room or area, the tripod stand can be removed and shifted to the next erection site. It serves only as an aid in assembling the slab formwork.

The tripod stand can be folded up to save space.



## Erecting the primary beams

Erection of the Variomax soffit formwork begins with setting up the primary beams.
For this, the props are set at roughly the required extension length on the ground. The fork heads are mounted to them, and then they are set up under the ends of the primary beams (in the case of jointed beams, under the joints as well). To keep them steady, tripod stands are attached to these props.

Following this, the H 20 beams are placed in the fork heads of the props. The assembly forks make this step much easier.

Then the remaining props should be set up, taking into account the static requirements (room height, slab thickness, and maximum permitted loading capacity of the tubular steel props which are being used).

The steel prop hangers which are attached to the props immediately safeguard them from falling over.

The prop is then swung into place under the primary beam.


### 4.0 Use and erection

## Placing the secondary beams

The correct distance between the secondary beams must be calculated, in line with the static requirements, by using the loading tables on pages 20 and 21.

A beam must be placed under each shuttering panel joint. Here, too, the work is facilitated by the assembly fork.

## Adding the shuttering panels

The shuttering panels are placed on top of the secondary beams and tacked in place. The rigid shuttering structure must be braced against the building.

## Note:

Safety rails must be erected on the edges of the structure, in line with the regulations for safety and health protection in shuttering and scaffolding $\mathrm{ZH} 1 /$ 603.

## Secondary beams and shuttering skin



Adding shuttering panels


The universal joint, with its simple and effective wedge connection, can be secured at any place on the H 20 beam.

It is equipped with a socket for the Combi-post.
The universal joint can also be used as a supporting bracket for shuttering the stop-end of a slab or an integrated beam.

The illustration shows shuttering of the stop-end of a slab, which is also a safety guard.
The Combi-post has 2 alternative positions.

## Universal joint and Combi-post



## Post position during shuttering work



## Post position on finished slab



### 4.0 Use and erection

## Joist clamping connector

Can be fastened to any timber beam like H 20 and R 24 with 8 cm wide and at maximum 6 cm thick flanges.

Adjustable fixing beam 500 This is fastened with the unlosable bolt to the joist clamping connector in steps of 1 cm corresponding to the required beam height. The position of the bolt is secured by means of a spring cotter

## Permissible distances

Joist clamping connectors are only to be arranged opposite on top of the same transverse beam.

The height " H " of the side formwork is decisive for the calculation. In case of having perimeter beams, the exterior side is decisive for the permissible distances.


## Erect the longitudinal main

 supports (1).Place transverse beams ( 2 ) on main beams.

Nail bottom shuttering skin ( 3 ) on beams ( 2 ).

Place and attach side shuttering skin (4).

Put on timber beam ( 5 ) or squared timber.

Position joist clamping connector ( 6 ) on top of the cross beam ( 2 ), press it against side formwork (4+5) and tighten wing nut by means of a hammer.

Fix the adjustable fixing beam ( 7 ) acc. to the desired height of beam formwork.
Place upper timber beam ( 8 ) on the angle iron welded to the fixing beam ( 7 ).

Erection of opposite side shuttering ( 9 ) after reinforcement work. Same procedure as already described before.

## Note:

Always arrange the joist clamping connector in opposite position on the same transverse beam.


### 4.0 Use and erection

## Joist clamping connector and

 Adjustable fixing beamCan be fastened to any timber beam like H 20 and R24 with 8 cm wide and at maximum 6 cm thick flanges.

## Examples of application

## Beam



Guard rail in conjunction with slab stopend shuttering


Slab stopend


The exterior side shuttering (here in this example) is formed by means of vertically arranged H 20 inserted in the C-profile of the joist clamping connector.
The application of the adjustable fixing beam is not required with this design.

## T-intersections and

 crossings of beamsEven in these areas it is possible to work with the joist clamping connector easily and without problems and without costly fitting work.
It is only to be noted that the transverse beams must not project more than 35 cm into the cross beams of the rectangularly arranged beam formwork of the intersection.

## Perimeter beam with slab connection


max.
35 cm

### 4.0 Use and erection

When joining timber beam slab formwork and beam formwork, two connecting variations arise through the directions of the secondary beams of the slab formwork.

Direction of secondary beam parallel to the beam formwork

The top timber beam H 20 of the side shuttering of the beam is arranged in such a way with the secondary beams of the slab formwork that it serves as support beam for the plywood of the slab at the same time.

The top timber beam H 20 of the side shuttering of the beam is lowered to such a degree that the secondary beam can directly be placed on this.

Other slab formwork systems can be bound into the beam formwork without any problems thanks to the height adjustment possibility of the adjustable fixing beam.

## Important note:

Maximum slab load per each joist clamping connector: 6.5 kN

## Compound use of slab and beam formwork



## Shuttering procedures

1. Secure fork head or drophead in the props.
2. Set up tripod stands (at least 4 in the corners of the room, plus additional units for primary beam joints.)
3. Secure the props in the tripod stands.
4. Lay the primary beams in the fork heads.
5. Attach additional props and steel prop hangers, in line with the table on page 12, under the primary beams.
6. Lay secondary beams.
7. Lay shuttering panels.
8. If necessary, set up auxiliary props under the infill panels, in line with DIN 1045.


### 5.0 Stripping

Stripping begins with lowering the props. For all tubular steel props from HÜNNEBECK, release bolts immediately reduce the pressure on the thread nut. A blow of the hammer suffices and then the slab shuttering can easily be lowered by about 6 cm turning down the adjustment nut.

Stripping procedures with fork heads 8/20:


The assembly fork is an effective tool for taking down the primary beams.
Remove the tripod stands from the props, and sort and stack all of the shuttering materials.

### 6.0 Stripping and transport aids

The shuttering materials are stored and transported in the practical Euro stacking frames. With the mobile set, which can be attached quickly, the materials can be brought within the range of the crane.
The Euro stacking frame is designed for a working load of $1,200 \mathrm{~kg}$. It can be moved with a crane, fork lift or the mobile set, which has a loading capacity of $1,300 \mathrm{~kg}$.
Up to 6 loaded frames may be stacked.

The two main wheels of the mobile set are easy to mount, simply by inserting their axles into the sockets in the stacking frame. A swing of the wheel lever lifts the stacking frame, and the swivel safety handle keeps the wheel lever in this position.
A supporting wheel which is inserted at the front of the stacking frame stabilizes the running gear.

## Storing and moving the shuttering materials



### 7.0 Tables for shuttering panels

## Variomax calculation

The existing slab thickness and the selected secondary beam spacing, which depends upon the type and size of the selected shuttering panel, determine the maximum permitted distance between primary beams.
Using the selected primary beam spacing and slab thickness, the maximum permitted distance between
the maximum permitted distance between props for the primary beam axes can then be determined.

All the figures necessary for the efficient use of Variomax soffit formwork can be quickly and precisely determined with the help of the following tables.


Table I

| Size of shuttering panel | Possible secondary beam spacing "e" |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 150/50 | $\begin{aligned} & \text { I I I } \\ & \mathrm{e}=75 \mathrm{~cm} \\ & \hline \end{aligned}$ |  |  | $\overline{\mathrm{I}}$ | $\begin{gathered} \text { I I I } \\ e=50 \mathrm{~cm} \\ \hline \end{gathered}$ |  |  |  |  |
| 200/50 |  | $\underset{e=66,7 \mathrm{~cm}}{\mathbf{I}}$ |  |  |  | $\begin{aligned} & \text { I I I } \\ & e=50 \mathrm{~cm} \\ & \hline \end{aligned}$ |  |  |  |
| 250/50 |  | $\begin{array}{lc} \hline \text { I } \quad \text { I } \\ \text { e }=62,5 \mathrm{~cm} \\ \hline \end{array}$ | $I$ |  | I |  | $\begin{gathered} \text { I I } \\ e=50 \mathrm{cr} \\ \hline \end{gathered}$ | $\begin{aligned} & \mathrm{I} \\ & \mathrm{~cm} \\ & \hline \end{aligned}$ | II |

Table II

| Max. secondary beam spacing <br> $[\mathrm{cm}]$ | Slab thichness [cm] <br> 22 mm <br> 3-ply boards |
| :---: | :---: |
| 75.0 | 20 |
| 66.7 | 25 |
| 62.5 | 30 |
| 50.0 | 62 |

Perm. deflection L/500.

Table III
Max. permitted span for $\quad-8 \rightarrow$ secondary beams L in „m"
= Max. primary beam spacing.

## Systems:

(Max. K = 0.50 m )

$\mp$ perm. L $\quad$


## Loading width for

## Tips for use:

Select slab thickness
Determine secondary beam spacing "e" (m)
taking into account type of shuttering panel (see page 20).
Determine max. span of secondary beam (this is equal to distances between primary beams).
Determine final distance between primary beams taking into account the allowable deflection of secondary beams.
Determine prop spacing for axes of primary beams taking into account the loading width (Edge section with/without cantilever and centre section).
a) Edge section beam without cantilever

b) or edge section beam
with cantilever in „m"


| (1) |  | (2) |  |  |  |  | (3) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\left\lvert\, \begin{gathered} \text { Slab } \\ \text { thickness } \end{gathered}\right.$[cm] | Total load $\left[\mathrm{kN} / \mathrm{m}^{2}\right]$ (*) | Secondary beam spacing [m] (unter Beachtung von Seite 20) |  |  |  |  | Gewählter Jochträgerabstand bzw. Belastungsbreite L [m] |  |  |  |  |  |  |  |  |  |  |
|  |  | 0.40 | 0.50 | 0.63 | 0.67 | 0.75 | 1.00 | 1.25 | 1.50 | 1.75 | 2.00 | 2.25 | 2.50 | 3.00 | 3.50 | 4.00 | 4.50 |
|  |  | Zul. Spannweite der Belagträger = max. Jochträgerabstand zul. L [m] |  |  |  |  | Zul. Stützenabstand unter Jochträgern [m] <br> (a, b oder c gem. Seite 11) |  |  |  |  |  |  |  |  |  |  |
| 10 | 4.35 | 3.99 | 3.71 | 3.44 | 3.37 | 3.24 | 2.94 | 2.71 | 2.48 | 2.29 | 2.14 | 2.02 | 1.92 | 1.69 | 1.44 | 1.26 | 1.12 |
| 12 | 4.87 | 3.78 | 3.51 | 3.26 | 3.19 | 3.06 | 2.78 | 2.56 | 2.34 | 2.17 | 2.03 | 1.91 | 1.81 | 1.51 | 1.29 | 1.13 | 1.00 |
| 14 | 5.39 | 3.60 | 3.34 | 3.10 | 3.04 | 2.92 | 2.65 | 2.44 | 2.22 | 2.06 | 1.93 | 1.81 | 1.63 | 1.36 | 1.17 | 1.02 | 0.91 |
| 16 | 5.91 | 3.45 | 3.21 | 2.98 | 2.91 | 2.80 | 2.54 | 2.33 | 2.12 | 1.97 | 1.84 | 1.65 | 1.49 | 1.24 | 1.06 | 0.93 | 0.83 |
| 18 | 6.43 | 3.33 | 3.09 | 2.87 | 2.81 | 2.70 | 2.45 | 2.23 | 2.04 | 1.89 | 1.71 | 1.52 | 1.37 | 1.14 | 0.98 | 0.86 | 0.76 |
| 20 | 6.95 | 3.22 | 2.99 | 2.77 | 2.71 | 2.61 | 2.37 | 2.15 | 1.96 | 1.81 | 1.58 | 1.41 | 1.27 | 1.06 | 0.90 | 0.79 | 0.70 |
| 22 | 7.47 | 3.12 | 2.90 | 2.69 | 2.63 | 2.53 | 2.30 | 2.07 | 1.89 | 1.68 | 1.47 | 1.31 | 1.18 | 0.98 | 0.84 | 0.74 | 0.65 |
| 24 | 7.99 | 3.04 | 2.82 | 2.62 | 2.56 | 2.46 | 2.24 | 2.00 | 1.83 | 1.57 | 1.38 | 1.22 | 1.10 | 0.92 | 0.79 | 0.69 | 0.61 |
| 26 | 8.51 | 2.96 | 2.75 | 2.55 | 2.50 | 2.40 | 2.17 | 1.94 | 1.72 | 1.48 | 1.29 | 1.15 | 1.03 | 0.86 | 0.74 | 0.65 | 0.57 |
| 28 | 9.03 | 2.89 | 2.68 | 2.49 | 2.44 | 2.34 | 2.10 | 1.88 | 1.62 | 1.39 | 1.22 | 1.08 | 0.97 | 0.81 | 0.70 | 0.61 | 0.54 |
| 30 | 9.61 | 2.83 | 2.62 | 2.44 | 2.38 | 2.29 | 2.04 | 1.82 | 1.53 | 1.31 | 1.14 | 1.02 | 0.92 | 0.76 | 0.65 | 0.57 | 0.51 |
| 40 | 12.73 | 2.57 | 2.39 | 2.22 | 2.17 | 2.05 | 1.73 | 1.38 | 1.15 | 0.99 | 0.86 | 0.77 | 0.69 | 0.58 | 0.49 | 0.43 | 0.38 |
| 50 | 15.85 | 2.39 | 2.22 | 2.01 | 1.95 | 1.83 | 1.39 | 1.11 | 0.93 | 0.79 | 0.69 | 0.62 | 0.56 | 0.46 | 0.40 | 0.35 | 0.31 |

(*) Total load assumed in the following way:

Weight of concrete
Concrete load
Dead load of formwork
Live load
Total load = Concrete load + Dead load of formwork + Live load
(**) Deflections of secondary beams are limited to:
$\mathrm{f}<\mathrm{L} / 300$ (first line) $-\mathrm{f}<\mathrm{L} / 500$ (second line)

## Prop loads

In most cases, the max. permitted prop spacing under primary beams, as stated in Table III, Column (3), results from the perm. shear load "Q" of the H 20 timber beam ( $2 \cdot 11.0 \mathrm{kN}=22.0 \mathrm{kN}=$ suport load for the steel prop). In this case, the following has to be considered:
If the permitted load "F" of the prop is less than 22.0 kN , the prop spacing under the primary beams should be reduced by the factor "max. perm.
F/22.0 kN".
(See also the example on page 14 under IV. Remark.)

## Note:

The maximum permitted prop load in relation to the extension length can be found in the Hünnebeck Folding Table with allowable loads for tubular steel props.

### 9.0 H20 Shuttering example

(see also load tables on pages 20 and 21)
Assuming loads as stated on page 20.

## I.) To be used for shuttering example:

| Clear floor-to-floor height | h | $=2.60 \mathrm{~m}$ |
| :--- | :--- | :--- |
| Slab thickness | t | $=16 \mathrm{~cm}$ |
| Selected beam |  | $H 20$ |
| Secondary beam spacing | e | $=0.75 \mathrm{~m}$ |
| Shuttering skin at hand |  | $=18 \mathrm{~mm}$ |

II.) Determining the max. perm. span for secondary beams

In Part (2) of Table III on page 21, find where the 16 cm slab thickness row intersects with the 0.75 cm secondary beam spacing column, to obtain the maximum permitted span of 2.85 m (= max. primary beam spacing with $f \leq L / 500$ ).
III.) Determining the max. perm. span for primary beams

In Part (3) of Table III, the first horizontal column shows the desired primary beam spacings and thus also the loading widths.

For example, for a room width of 3.50 m , primary beams RJ1 and RJ2 (see ground-plan) have a loading width of 1.75 m . By seeing where the 1.75 m column intersects with the 16 cm slab thickness row, you obtain the max. prop spacing of 2.0 m for the edge section beam.
For the centre section beam MJ , taking into account the loading span of 2.5 m , the max. permitted prop spacing is calculated at 1.53 m (= max. prop spacing).

The applicable HÜNNEBECK tubular steel props Folding Table shows the following figures. Given an actual clear room height of 2.60 m , and subtracting $2 \cdot 20 \mathrm{~cm}$ beam height +18 mm shuttering skin, we arrive at a prop extension length of 2.18 m .
We meet the requirements of the prop load tables, for example, and find, at 2.20 m extension length for the „Europlus 260 DB/DIN" a max. permitted load of $F=33.0 \mathrm{kN}>22 \mathrm{kN}$.

## Remark:

Since the max. permitted ' F ' of the prop is more than 22 kN , the prop spacing of the primary beams is not to be reduced.

This means: $2 \cdot \mathrm{Q}=\underline{22 \mathrm{kN}}$ for H 20 beam $<33.0 \mathrm{kN}$
An additional example: Clear room height $=3.00 \mathrm{~m}$ : $3.00 \mathrm{~m}-0.42 \mathrm{~m}=2.58 \mathrm{~m}$ total prop length permitted load for Europlus $260=\underline{26.0 \mathrm{kN}}>22.0 \mathrm{kN}$.
20.0 kN perm. 22 kN exist.
$=0.91 \mathrm{kN}$
$R J=$ edge section beam
$M J=$ centre section beam
$0.91 \cdot 1.93 \mathrm{~m}=$
1.76 m perm. prop spacing

(As per Table III)

## Materials summary

(example)

| No. | Description | Art.No. |
| ---: | :--- | :--- |
| 2 | H $20-245$ | 581770 |
| 6 | H $20-290$ | 581792 |
| 13 | H $20-330$ | 581807 |
| 4 | H $20-450$ | 581830 |
| 14 | Props 260 DB/DIN | 463021 |
| 9 | Fork head 8/20 | 417565 |
| 5 | Steel prop hanger | 510749 |
| 9 | Tripod stand | 510256 |
|  | Suggested: |  |
| 14 | T-bolt | 470804 |
| 2 | Assembly fork | 510554 |
| 2 | Euro stacking frame | 553689 |
| 1 | Mobile Set | 563722 |

## Perm. load [kN]

DIN 4421, falsework clase III

| as per Euronorm <br> DIN Standards |  |  |  | $\begin{aligned} & \underline{Z} \\ & \overline{0} \\ & \mathbf{N} \\ & \text { o } \\ & \stackrel{0}{n} \end{aligned}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Size | 1 | 2 | 3 | 4 | 5 |
|  |  |  |  | $\begin{array}{rc} 1 & \varepsilon \\ \infty & 0 \\ 0 & 0 \\ -i & 0 \\ \hline \end{array}$ |  | $$ |
|  | 1.50 | 30,00 |  |  |  |  |
|  | 1.60 | 30.00 |  |  |  |  |
|  | 1.70 | 30.00 | 30.00 |  |  |  |
|  | 1.80 | 30.00 | 30.00 |  |  |  |
|  | 1.90 | 28.81 | 30.00 |  |  |  |
|  | 2.00 | 26.00 | 30.00 | 30.00 |  |  |
|  | 2.10 | 23.58 | 27.21 | 30.00 |  |  |
|  | 2.20 | 21.49 | 24.79 | 28.93 |  |  |
|  | 2.30 | 20.00 | 22.68 | 26.47 | 30.00 |  |
|  | 2.40 | 20.00 | 20.83 | 24.31 | 28.47 |  |
|  | 2.50 | 20.00 | 20.00 | 22.40 | 26.24 | 28.80 |
|  | 2.60 | 20.00 | 20.00 | 20.71 | 24.26 | 26.63 |
|  | 2.70 |  | 20.00 | 20.00 | 22.50 | 24.69 |
|  | 2.80 |  | 20.00 | 20.00 | 20.92 | 22.96 |
|  | 2.90 |  | 20.00 | 20.00 | 20.00 | 21,40 |
|  | 3.00 |  | 20.00 | 20.00 | 20.00 | 20.00 |
|  | 3.10 |  |  | 20.00 | 20.00 | 20.00 |
|  | 3.20 |  |  | 20.00 | 20.00 | 20.00 |
|  | 3.30 |  |  | 20.00 | 20.00 | 20.00 |
|  | 3.40 |  |  | 20.00 | 20.00 | 20.00 |
|  | 3.50 |  |  | 20.00 | 20.00 | 20.00 |
|  | 3.60 |  |  |  | 20.00 | 20.00 |
|  | 3.70 |  |  |  | 20.00 | 20.00 |
|  | 3.80 |  |  |  | 20,00 | 20.00 |
|  | 3.90 |  |  |  | 20.00 | 20.00 |
|  | 4.00 |  |  |  | 20.00 | 20.00 |
|  | 4.10 |  |  |  | 20,00 | 20.00 |
|  | 4.20 |  |  |  |  | 20.00 |
|  | 4.30 |  |  |  |  | 20.00 |
|  | 4.40 |  |  |  |  | 20.00 |
|  | 4.50 |  |  |  |  | 20.00 |

## Defined factors for calculations:

## A. Statical figures

H2O timber beam
perm. $\mathrm{M}=5.00 \mathrm{kNm}$
perm. $Q=11.00 \mathrm{kN}$
$\mathrm{E} \cdot \mathrm{I}=500 \mathrm{kNm}^{\mathbf{2}}$

B. Dimensions
C. Material requirements and weights:

| H 20 beams | 3 | $\mathrm{~m} / \mathrm{m}^{2}$ | $15.0 \mathrm{~kg} / \mathrm{m}^{2}$ |
| :--- | :--- | ---: | ---: |
| Tripod stands | 0.3 | $\mathrm{pcs} . \mathrm{m}^{2}$ | $3.3 \mathrm{~kg} / \mathrm{m}^{2}$ |
| Fork heads $8 / 20$ | 0.3 | $\mathrm{pcs} . / \mathrm{m}^{2}$ | $0.1 \mathrm{~kg} / \mathrm{m}^{2}$ |

Props used for housing construction:
e.g. Europlus 260 DB/DIN 0.5 pcs./m
$7.8 \mathrm{~kg} / \mathrm{m}^{2}$

## D. Time figures for erection / striking:

The expenditure of time for the erection and striking of the formwork depends on the ground plan and dimensions of the building: Variomax Soffit Formwork: $\quad 0.3-0.5 \mathrm{~h} / \mathrm{m}^{2}$

## E. Transport volume of H 20 timber beams

Weight taken from item (C) in tons multiplied by 3.5 results in the required transport volume $\left[\mathrm{m}^{3}\right]$.


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